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Study of the Durban Bight shoreline evolution under schematised climate change and sand-bypassing scenarios

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The Durban Bight beaches, on the KwaZulu-Natal coast of South Africa, have been the topic of several studies in the past, because of their recreational and economic value. These beaches are mainly nourished via a sand-pumping scheme, which has been the case since the 1980s. The present study investigated the longer-term dynamic stability of the Durban Bight beaches using a one-dimensional numerical shoreline model. The wave conditions that drive the longshore transport of sand were simulated using the numerical spectral wave model Simulation WAVes in the Nearshore (SWAN). The shoreline model was calibrated and validated against existing historical data. The calibrated model was then employed to simulate possible future scenarios. These included climate change phenomena and the influence of anthropogenic changes. Each of the simulations predicts a twenty-year period, with results extracted at five-year intervals. The results are presented and discussed, and the emphasis is placed on the importance of maintaining the required sand-bypassing rates and the awareness of the possible effects of climate change on shoreline maintenance and management.

INTRODUCTION

Background

The Durban Bight, on the east coast of South Africa in the KwaZulu-Natal Province, has been the topic of numerous studies in the past (Mather *et al* 2003; Cawthra *et al* 2012; Campbell *et al* 1985; Soltau & Smith 2003; Mather & Stretch 2012; Green *et al* 2012; Habets 2015). Durban Harbour is one of South Africa's and East Africa's largest harbours, which mainly functions as import and export harbour for containers, oil and food (Rossouw & Theron 2012). Import and export container throughput per annum is estimated at 41.9 Megatons (2.48 Million Twenty-foot Equivalent Units (MTEU)) (Rossouw & Theron 2012). Durban Harbour is the largest non-bulk terminal in South Africa, and up to 2010 was also the second-busiest port in the southern hemisphere (eThekweni Municipality 2010). In close proximity to the harbour, Durban also boasts some of South Africa's most well-known recreational beaches, which attract both national and international tourists. Due to this hotspot of coastal activity, the Durban Bight forms an ideal study location for various aspects of natural and anthropogenic changes

(Mather & Stretch 2012; Theron 2007; Mather *et al* 2003; Barnett 1999).

One of the most significant alterations to the natural beaches in the Durban Bight is the sand-bypassing scheme located at the harbour entrance. This scheme was implemented to compensate for the loss of sediment transported from the south to the north, due to the construction of the Durban Harbour southern breakwater. The longshore sediment transport from the south was cut off, resulting in severe erosion of the Bight beaches located to the north of the harbour entrance, prior to the 1980s. Currently the beaches are mainly nourished via the sand-bypassing scheme consisting of a sand trap (just south of the breakwater) and a dredger that pumps sand into a land-bound hopper system that transports sediment in a fluidised state to the Bight beaches. The sand-pumping pipelines consist of approximately 3.5 km of conduits (Mather *et al* 2003). A schematic illustration of this scheme is presented in Figure 1. With historical data of the sand-pumping volumes, this anthropogenic influence on the Durban Bight beaches can be studied and modelled.

The focus of the present work is on the long-term shoreline dynamics of the Durban Bight associated with:

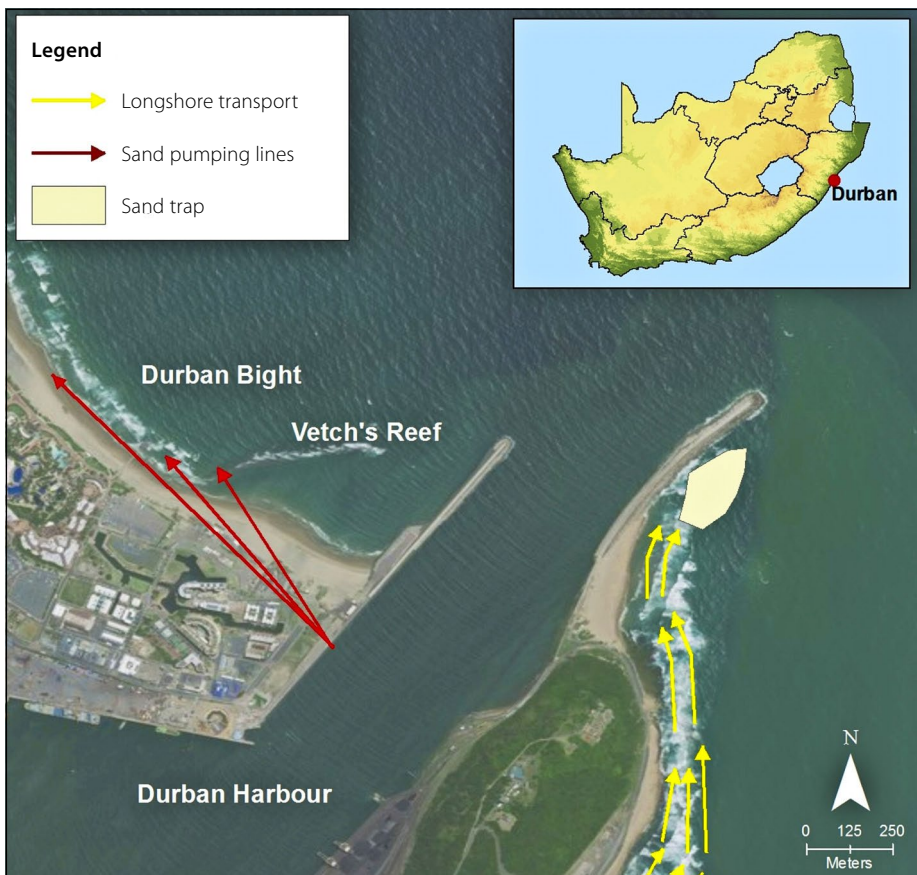


Figure 1 Image of Durban Harbour, indicating the locations of the sand trap, harbour entrance and Durban Bight, with (inset) a map of southern Africa showing the location of Durban

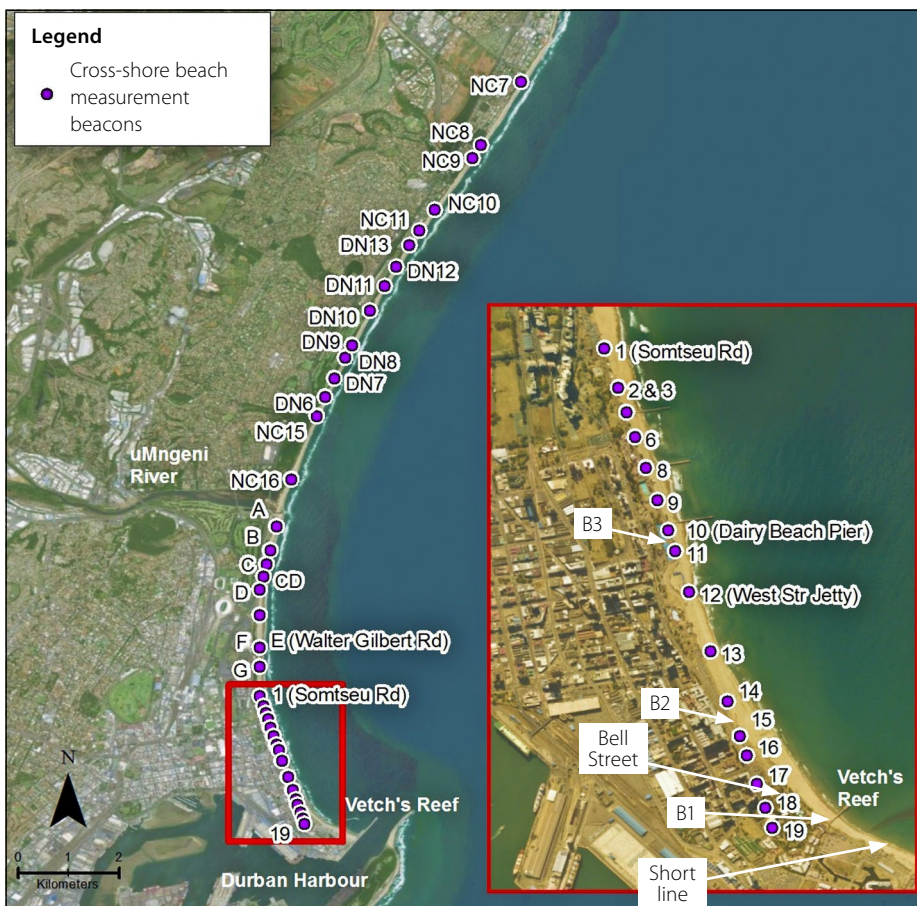


Figure 2 Layout of the greater Durban Bight indicating the beach surveying beacon locations and their names; this illustration also indicates the extent of the numerical model employed for the current study (Vetch's Reef to beacon NC7) and the sand-bypassing outlet locations in the zoomed-in view around Vetch's Reef

- global climate change
- the possible construction of a new dig-out harbour south of the existing harbour (Habets 2015)
- changes in sand-bypassing volumes and sediment characteristics, and
- the effect of dam building in rivers that mouth south of the harbour.

These factors can influence the quality of the Durban beaches significantly, and investigating these scenarios will contribute to effective planning and coastal hazard assessment by the eThekweni Municipality (greater Durban area) (Mather & Stretch 2012). In Figure 2 the layout of the greater Durban Bight is illustrated, together with the cross-shore measurement beacon locations utilised in the present study for calibration and validation. Relatively limited studies have been performed regarding climate change in southern Africa and the associated vulnerability and mitigation options, with the exception of some studies in South Africa (Theron 2007; Mather & Stretch 2012; Theron *et al* 2012). The present study will supplement existing studies to quantify the influence of certain future scenarios on the long-term shoreline dynamics in the Durban area, with the aim to inform local coastal management.

With knowledge of such potential changes, the eThekweni Municipality will be able to investigate ways to manage and mitigate potential impacts of wider or narrower beaches, such as blockage of stormwater outlets, wind-blown sand problems, reduced available beach space for recreational activities and possible effects on the Umgeni River mouth.

Aim

The aim of this study is to use a long-term shoreline model to provide updated predictions of the potential future shoreline changes resulting from four future scenarios. To this effect the following approaches were followed:

- Employ the Simulation WAVes in the Nearshore (SWAN) wave propagation model to transform offshore wave conditions to the nearshore.
- Calibrate a Unibest (Deltares 2005) shoreline model, employing 200 to 400 representative wave conditions.
- Use a model domain extending up to Umhlanga Rocks (Beacon NC7) in the north.
- Include in the shoreline model the sediment inputs from the Umgeni River and the beach nourishment scheme.

The model will also be employed to investigate possible effects on the shoreline due to:

- Climate change
- The construction of a new port
- The construction of dams in the rivers feeding the central KwaZulu-Natal coast
- Changes in the particle size distribution (PSD) of the dredged sand.

To be able to predict the shoreline dynamics accurately the representative wave conditions had to be analysed first. This includes the setting up of the wave transformation model, analysis of wave data and determination of the conditions that are most representative for the Durban Bight. The predictions of potential future shoreline responses to the future scenarios are provided and discussed. Conclusions are made and recommendations are given.

SHORELINE MODEL SET UP

Model description

The Unibest CL+ numerical shoreline model (Deltares 2005) was employed in this study to simulate shoreline evolution based on wave-driven longshore transport. The model incorporates two sub-models that run in tandem: In the Unibest LT sub-model, linear nearshore wave refraction, shoaling and breaking are predicted. The resulting longshore current and sediment transport are then calculated. This information serves as input (at several locations along the shoreline) to the Unibest CL sub-model, with which one-dimensional shoreline evolution is simulated. The latter sub-model allows for the incorporation of varying sediment sources, such as the sand-feeding along the Durban beaches from natural and anthropogenic sources.

The shoreline model functions based on single-line theory, which assumes the beach and nearshore profile to be represented by a cross-shore profile shape that does not change in time (Bosboom & Stive 2015). As the shoreline erodes or accretes, the entire equilibrium cross-shore profile moves either landward or seaward (Bosboom & Stive 2015). The model is ideal for medium- to long-term predictions of shoreline evolution. Generally, the model is run with a climate of representative wave conditions. The model can also simulate the temporal response of the shoreline to varying wave conditions. For periods shorter than seasonal scale, observed shoreline behaviour is dominated by on/offshore processes (storms/calm periods).

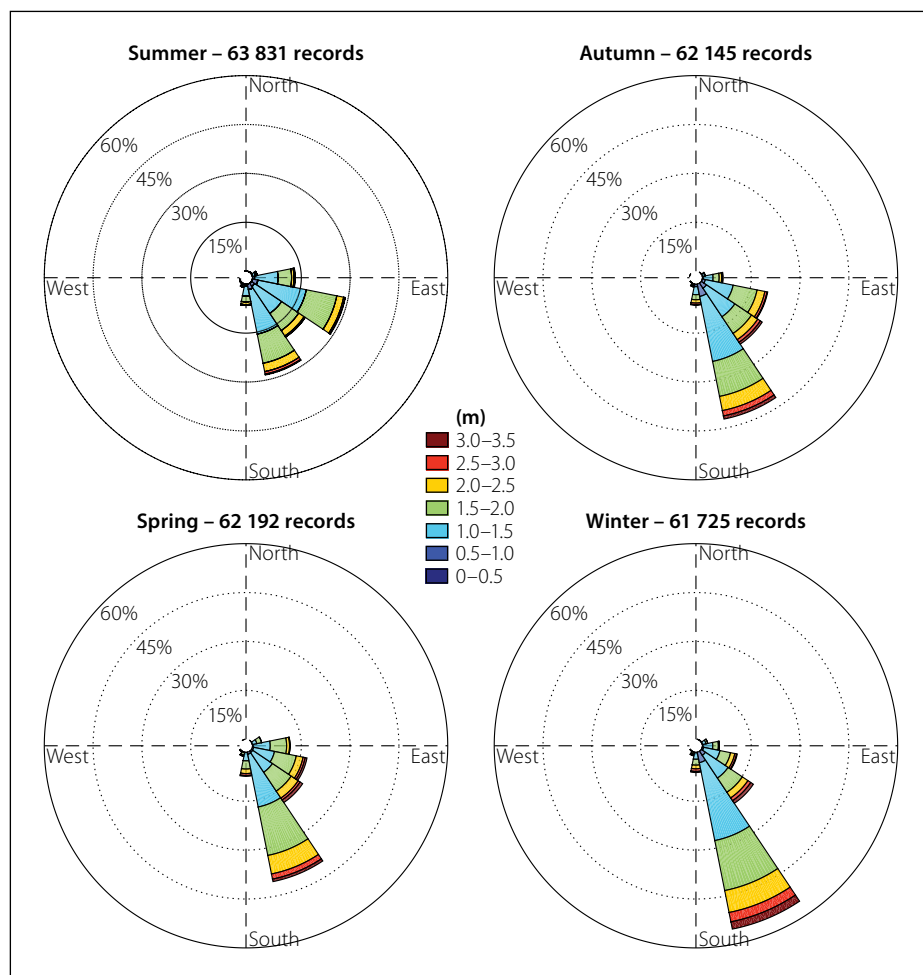


Figure 3 Seasonal wave roses for the directional wave rider buoy records at Richards Bay (at 28.8265 S, 32.104 E), in 23 m water depth; these roses represent all the data from 8 November 2002 to 23 October 2013

Three-dimensional effects are not included in the model formulation and therefore the cross-shore erosion/accretion, due to e.g. storms (episodic erosion), is not represented in the model process. In a shoreline model, such factors are assumed to average out over the long time periods (Hugo 2013). The model is thus appropriately used to simulate structural erosion/accretion and not episodic erosion associated with the cross-shore movement of sediments (Bosboom & Stive 2015).

Wave input

During the shoreline model calibration period (1990 to 2006) waves were recorded off Durban by means of a Waverider buoy (1992–2001) and an Acoustic Doppler Current Profiler (ADCP 2002–2007) (Rossouw *et al* 1999). However, the older wave rider recordings off the Durban Bluff lacked wave directions, while the shallow ADCP wave data suffered from intermittent coverage and accuracy issues due to various challenges, for example episodic sand inundation of the instrument (Mather & Theron 2011).

These wave data sources were evaluated and transformed from their in-situ location to the offshore model boundary location. Transformation tables were used to avoid simulating recurring conditions. Datasets were transformed back and forth from these measurement locations to the offshore model boundary to assure the accuracy of the transformation tables. A near 1:1 comparison was obtained between the twice-transformed wave conditions and the original measurement datasets.

After transforming several wave datasets to the offshore model boundary, as a virtual time series, it was found that the most representative transport rates are provided by the data from the directional wave buoys at the Port of Richards Bay. In Figure 3 the seasonal directional wave roses at Richards Bay are given. The predominant wave direction is from the south-southeast with a directional spreading difference between the various seasons. Summer was the only season presenting a stronger east-southeast wave directional component.

In the Bight an approximate annual sediment transport rate of 265 000 m³ can

be expected 305 000 m³ further north of the Umgeni River mouth (Theron 2015; Wells 2015) and the net north-eastward annual longshore sediment transport rate of about 500 000 m³ along the Durban Bluff (Schoonees 2000; Theron & Rautenbach 2014). Both resulting transport directions and magnitudes compare well to the known estimates, and the nearshore refracted wave parameters were confirmed with the nearshore ADCP dataset. Transforming only local ADCP data from the Bight failed in reproducing the wave conditions offshore (and thus nearshore transport rates) for the greater extent of the shoreline, due to a dominant easterly wave component. The peak wave period, significant wave height and directional time series correspond to the ADCP data less than a kilometre away from the tip of the Durban southern breakwater, in 17 m water depth (the data was provided by the local port authorities) (Diedericks *et al* 2011). This data was therefore utilised in the present study. The Richards Bay wave buoy was in a water depth of 23 m.

Wave conditions were extracted from the 437 refraction simulation outputs, just seaward (between 8 m and 6 m, depending on wave exposure) of wave breaking. The SWAN model consisted of three computational grids. The parent domain had a resolution of 1 km, the intermediate grid resolution was approximately 150 m and the final coastal nested grid resolution was approximately 20 m. The coastal nested domain was a curvilinear grid, while the other two domains were rectangular. The parent domain covered approximately 150 km along the coastline and the offshore boundary of the parent grid was approximately 60 km away from the coastline.

To ensure that alongshore changes in wave conditions are represented throughout the Bight, waves were input to the shoreline model at 48 locations. Approximately half of these were located north of the Umgeni River and half south of this point (refer to Figure 2). This gave greater computational density in the area of greater wave and bathymetric variability, with lower density along the northern part where the shoreline and bathymetry are fairly uniform. Transport conditions were interpolated between locations in the shoreline model.

Boundaries

The modelled domain extends from the present Vetch's Reef (or pier) in the south to Umhlanga Rocks in the north, a distance

of approximately 16 km (refer to Figure 1). The coastline between Vetch's Reef and the North Breakwater of the port was not included in the model, as the focus of the study is on beach accretion of the main beaches (and secondarily because modelled wave conditions are less accurate in this partial diffraction zone).

At the boundaries of the model the transport (or shoreline behaviour) needs to be specified. No sand enters the Vetch's Bight area from the south, due to the intervening harbour breakwaters and deeper dredged entrance channel. At the southern model boundary, a variable input of sediment was used, this being the combined historical discharge rates, from the so-called Short Line and Long Line, which outlets formed part of the beach nourishment scheme. Both discharge close to Vetch's Reef, in the corner of the Bight, close to the harbour entrance. This was assumed to be a reasonable discretisation, as the Vetch's Bight cell appears to be neither a substantial sink nor a source of sand (CSIR 2003; Theron & Rautenbach 2014). Discharged sand, e.g. into the Bight by means of the Short Line, is therefore likely to exit northward towards the Durban beaches within a reasonably short period of time.

The northern boundary of the model was located at Umhlanga Rocks. At this point the rocky coastline forms a rocky headland protruding from the sandy beach that lies to the south, forming a non-erodible coast with an embayment to the north. Northward transport can thus readily occur while southward return transport is restricted by the embayment and headland, as well as by the local coastal orientation relative to the incident wave directions resulting in virtually unidirectional longshore currents and transport towards the north. This situation is reproduced by schematising the model boundary with a short groyne at Umhlanga Rocks and a fixed northward transport on its northern side (implying no southward transport across the groyne). Transport rates south of the boundary are influenced only by the transports and shoreline behaviour occurring in the remainder of the model domain. These determine the actual transport past the groyne and thus the model boundary.

Cross-shore profiles

Beach and nearshore profiles are required in the shoreline model to calculate wave transformation and longshore transport processes. The nearshore profiles were

therefore extracted from the bathymetry. Due to some uniformity in the nearshore bathymetry, the profiles were schematised to six typical profiles based on measurements provided by the eThekweni Municipality.

Sediment size

Limited samples are available of the material dredged from the sand trap. From 2000 to 2004, samples from the sand-pumping scheme hopper indicated a profusion of coarse material. This was in contradiction to previous samples (1999), which indicated an average of 253 μm , once outliers were removed. Earlier sampling indicated sizes between 200 and 250 μm (Campbell *et al* 1985). The long-term averages, of dredging samples collected near-annually by eThekweni Municipality, indicate D_{50} values in the range between 250 μm and 300 μm (Theron *et al* 2013). A value of 250 μm was thus applied as an approximation in the shoreline model calibration, validation and baseline scenario. For the future scenarios two extreme cases were investigated – one with D_{50} a third less than the baseline scenario (166 μm), and one with D_{50} a third more than the baseline scenario (333 μm).

Model effective depth

The model effective depth parameter, or active height of the profile, describes the height of the mobile sand layer that responds to changes in shoreline orientation. Based on previous modelling investigations at the Council for Scientific and Industrial Research (CSIR 2007), a variable depth was used, ranging from 10 m in the north to 6 m in the more sheltered extreme south. During the predictions of future shoreline evolution, this parameter was increased to account for areas where material would be deposited into deeper water if the shoreline accreted substantially. It should be noted that the model effective depth has virtually no influence on the configuration of the long-term equilibrium shoreline, only the rate at which shoreline equilibrium is attained in response to a steady rate of sand discharge.

Sediment input from the Umgeni River

Data on the amount of sediment discharged to the sea by the Umgeni River is poor. However, the present typical annual input is very low, as there are limited sources of sandy material below the Inanda Dam (Garland & Moleko 2000). Sediment inputs occur mostly during episodic

events. It has been estimated that the last extreme flood (1987) discharged between 720 000 m³ and 1.24 million m³ of sand. It is assumed that the present annual discharge is unlikely to be more than a few percent of this, due to five large dams trapping virtually all of the sediments (Theron *et al* 2008).

Longshore transport rates calculated north and south of the Umgeni area indicate a difference in the order of 70 000 m³/a (Soltau & Theron 2007). However, shoreline changes (net erosion) north and south of the mouth suggest that at present this magnitude of material is not being supplied from river inputs (i.e. the difference in transport is being supplied by beach erosion). Tests with the shoreline model indicate that it is fairly insensitive to the input rate. The calibration suggested that a sediment input in the order of 20 000 m³/a was appropriate. This represents the volumes contributed by the river itself and any remnant contribution from sub-tidal flood deposits. This quantity was applied for the simulations.

SHORELINE MODEL CALIBRATION AND VERIFICATION

With the purpose of the study being to simulate future shoreline changes, the shoreline model was calibrated against actual measured shoreline changes. The model was set up with the appropriate sediment sources to represent the historic sand-pumping rates, and calibrated against measured shoreline changes at the municipality's beach survey beacons (which data is sourced from the eThekweni Municipality's extensive coastal monitoring programme, as for example reported on in Theron *et al* (2013).

Model calibration

The period of model calibration was the sixteen-year period from 1990 to 2006. This period was selected as the groynes/piers were in place (constructed 1982 to 1988) and the sand-pumping scheme had been in operation for long enough (since 1982) to allow the shoreline to adapt to these changes. Comprehensive survey and pumping data was available (through eThekweni Municipality's coastal monitoring programme, e.g. Theron *et al* (2013)).

Discharge data

From 1990 to 2006 a volume of approximately 4 207 000 m³ was pumped onto the beaches at an average annual rate of

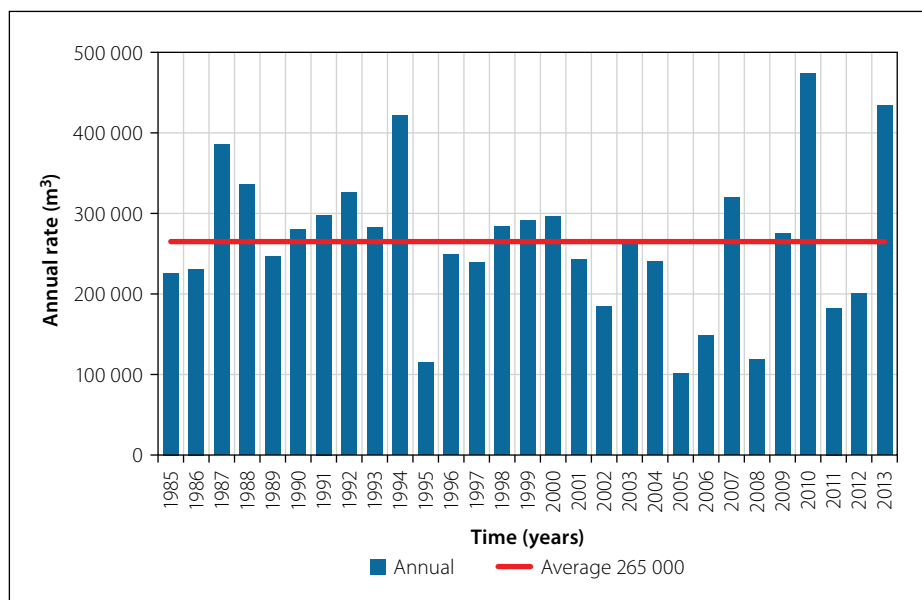


Figure 4 Longterm total sand-pumping rate in cubic metres (Theron & Rautenbach 2014)

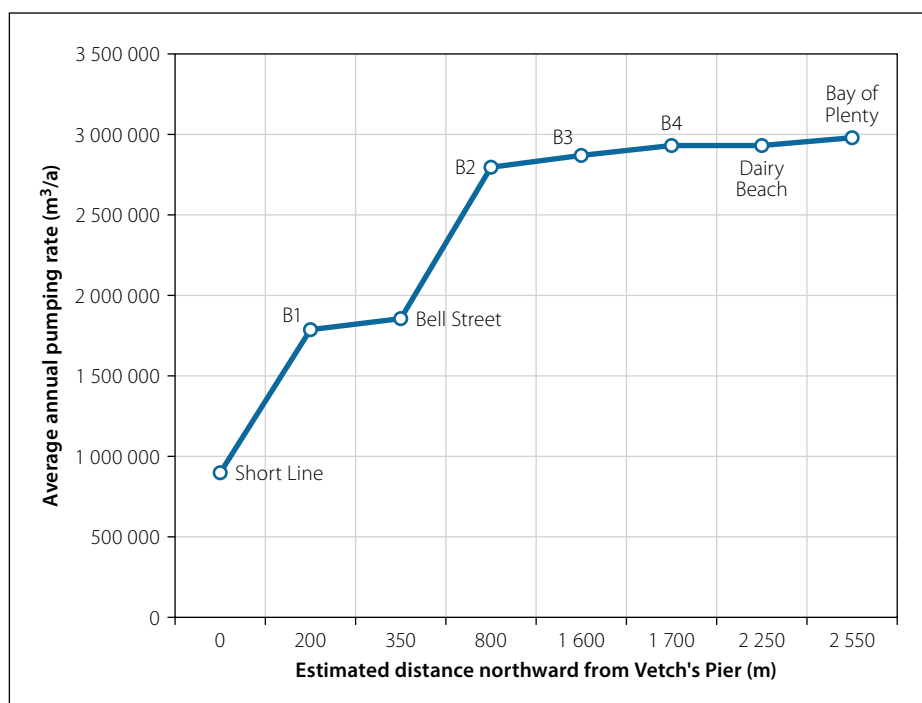


Figure 5 Average sand discharge rates along the shoreline from 1989 to 2005

265 000 m³. The lowest annual pumping rate during this period was 99 000 m³ (2005) and the highest was 444 000 m³ (1994). The average rate has varied considerably over time. The running average annual rate is illustrated in Figure 4 for the period 1989 to 2013. During this period, the average annual rate has been 265 000 m³. In general, the annual rate has been decreasing in the past three decades.

The average distribution of pumping volumes between the different beach outlets is given in Figure 5, noting that the Short Line and Long Line volumes have been combined in this figure, as this is how they were applied in the model (refer to Figure 2). Most of the sand (almost 90%)

has been discharged south of the Dairy Beach outlet.

Calibration results

The calibration results are depicted in Figures 6(a) to (f). In these figures the measured horizontal offset data of the +2 m Chart Datum (CD) contour is depicted against the simulation model results (also at the +2 m CD contour) for the period of interest. The results depicted are given only for some of the beacon locations along the Bight (refer to Figure 2).

The accuracy of the calibration results was determined via the Root Mean Square Error of Prediction (RMSEP), which is defined as:

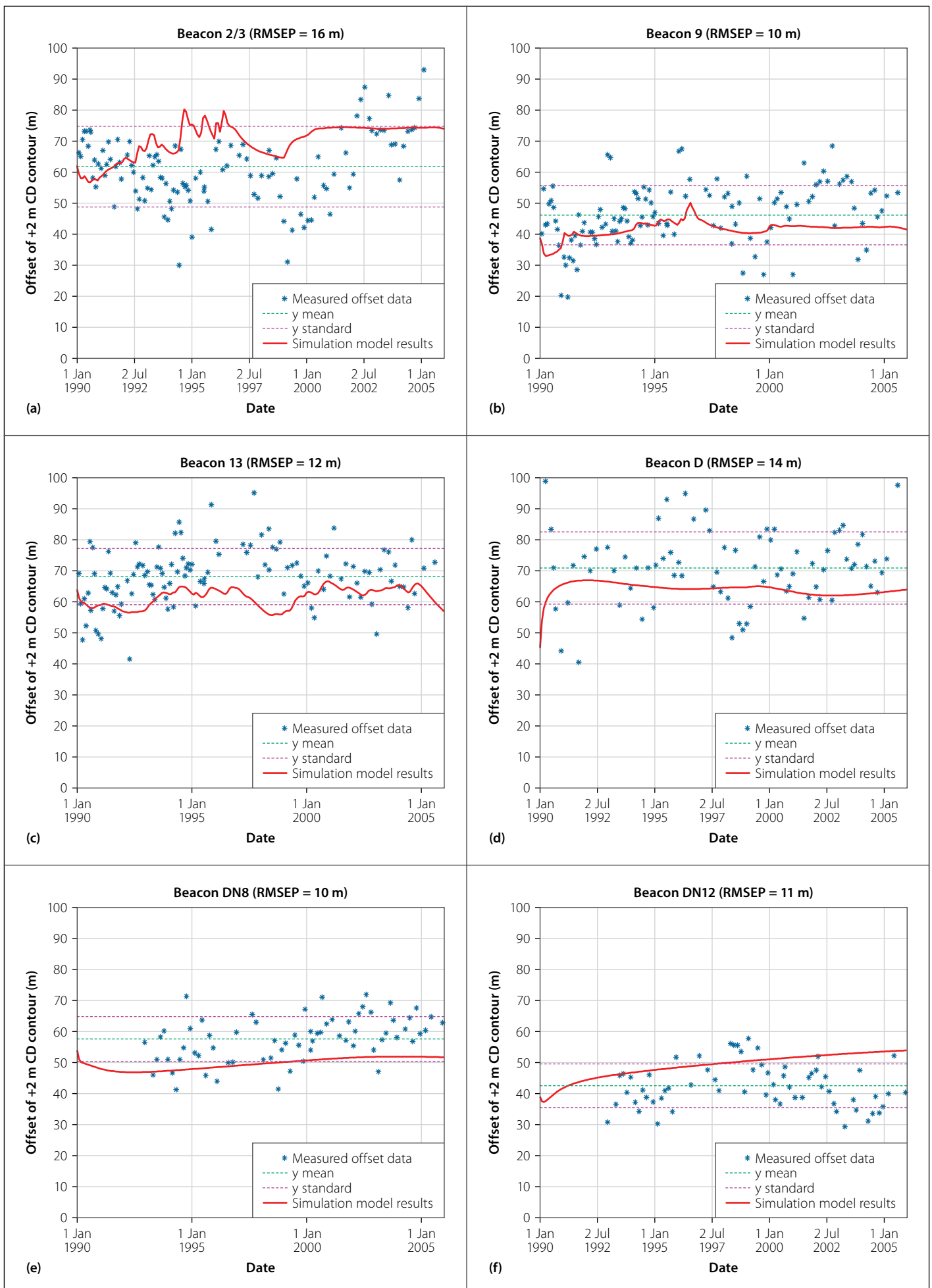


Figure 6(a)–(f) Calibration results for intermittent beacons along the Bight; each result indicates the measured data, simulation results and the mean and standard deviation of the measured data (refer to Figure 2)

$$RMSEP = \sqrt{\frac{\sum_{i=1}^n (\hat{y}_i - y_i)^2}{n}}$$

Where \hat{y}_i is the predicted values from the model, y_i is the measured data values and n is the number of measured data points.

The RMSEP is also given in metres and is thus an easy-to-understand method of quantifying the model prediction error. It should be emphasised that the Unibest model does not consider cross-shore effects. This is also clear in the comparisons presented in Figure 6. The variation in the measured data can, to a large extent, be attributed to cross-shore effect (for example storm events). The total average long-term behaviour is, however, dominated by the longshore transport of sediment. Thus, the ability of the model to predict the long-term behaviour of the beaches is adequate. It is also important to notice that the RMSEP value varies as the standard deviation of the measured data varies. The

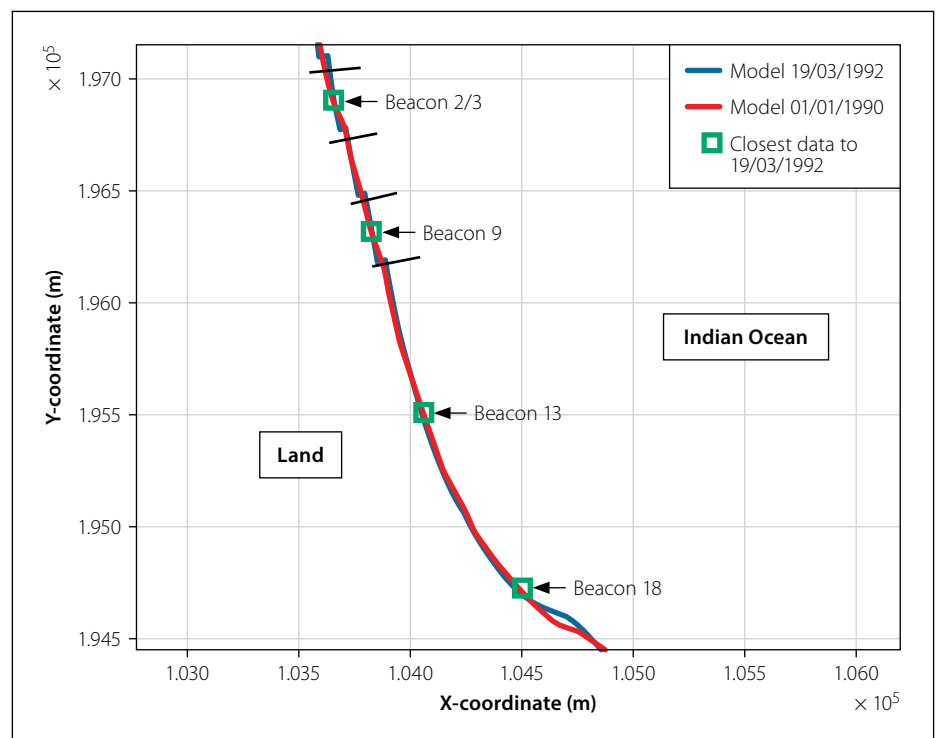


Figure 7 Area of the Durban Bight close to the piers; the model results at 19.03.1992 and 01.01.1990 are presented together with the data points measured closest to the date 19.03.1992

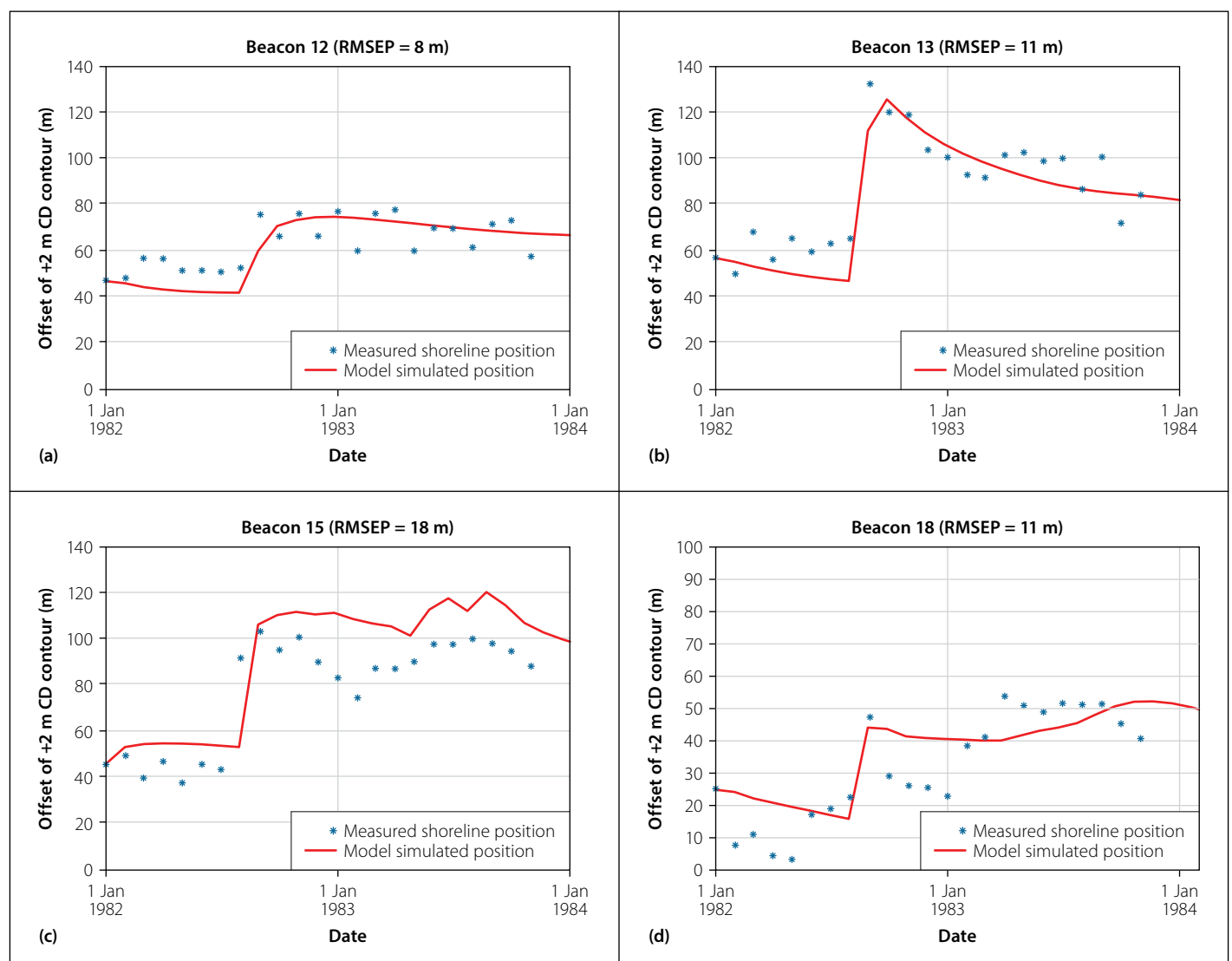


Figure 8(a)–(d) Validation results for intermittently measured beacons between the harbour entrance and the piers; each result indicates the measured cross-shore data along with the simulation results

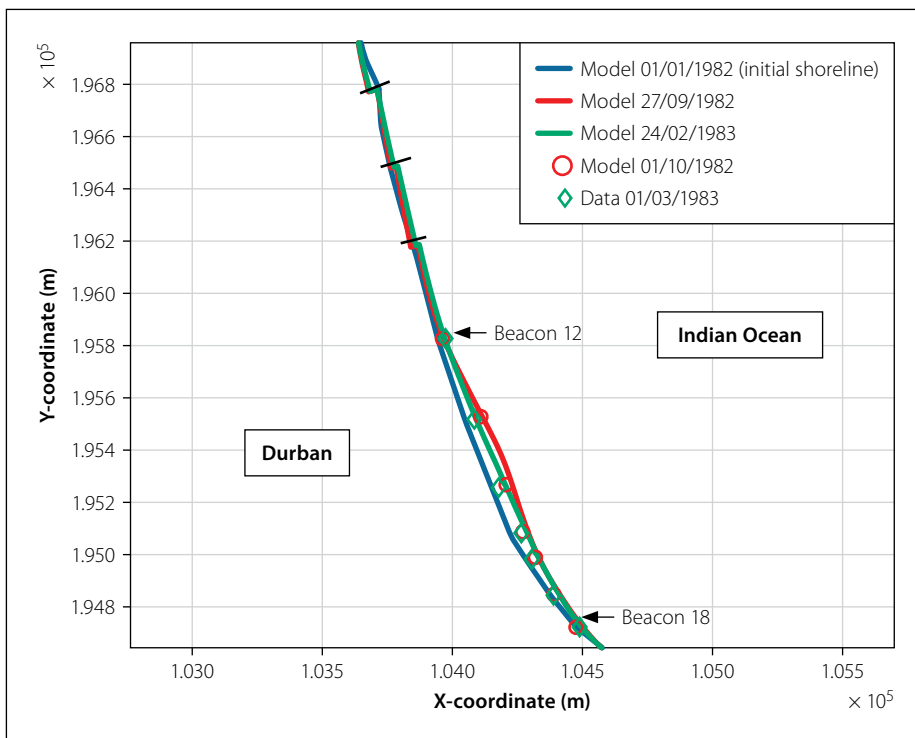


Figure 9 Plan view of some of the validation results; measured data is compared with model results

accuracy of the model prediction is thus directly related to the spread in the data. A large spread in the measured data is also an indication of a more dynamic beach regarding cross-shore processes. To resolve these processes a fully three-dimensional or cross-shore model would have to be employed over a period usually shorter than seasonal scale.

A shoreline model with an RMSEP of less than 10 m can be regarded as extremely accurate. Figure 7 is provided to illustrate the results of Figure 6 in a more realistic perspective. Here it can be seen clearly that even discrepancies that seem large in Figure 6 are in fact adequate in predicting shoreline variation. The South African Lo 31 WGS84 coordinate system was used in the model, with the following transformation to local model coordinates: $X_{\text{model}} = 100\,000 - Y_{\text{Lo31}}$ and $Y_{\text{model}} = 3\,500\,000 - X_{\text{Lo31}}$.

Model validation

Validation of a model calibration provides a check on the model accuracy. Such validation data should ideally be of a similar nature to that for which the model is to be applied, i.e. similar shoreline responses or discharge rates.

Discharge data

In 1982 a large nourishment of South Beach (approximately Beacon 13) and Addington Beach (approximately Beacon 6) was carried out by a contracted dredging

company. A sand volume of approximately $600\,000\text{ m}^3$ was placed on the beach in a period of six weeks between August and September 1982. Limited data is available on the precise location or rate of nourishment. The shoreline model was set up to simulate this nourishment event. During the period from July 1982 to November 1983, routine sand nourishments also occurred, amounting to $266\,000\text{ m}^3$. These were also included in the model.

Regular surveys of the nourishment area were conducted. The shoreline accreted by between 30 m and 80 m. The surveys were used to validate the model predictions of the shoreline accretion resulting from the nourishment.

Validation results

Similar to Figure 6 the results of the model validation are presented in Figures 8(a) to (d). The RMSEP was utilised to quantify the model prediction accuracy. Not only did the model simulate the large nourishment event accurately, but it also indicated acceptable RMSEP model correlations. In Figure 8 only beacons between the harbour entrance and the piers are given, as this was the area with the largest changes in shoreline location.

The best way to illustrate the accuracy of a longshore transport model is by viewing the results in the form of scaled shoreline changes. In Figure 9 the model results are compared with the measured data. Two dates were chosen which correspond

best with the large changes in shoreline location. The conclusion is that the model performs adequately and can thus be used to simulate predictions of possible future scenarios.

FUTURE SCENARIOS

The first and most probable future scenario is the decrease in the amount of sand pumped onto the Durban beaches. There can be many reasons for the decrease in sand supply and the change in sediment characteristics:

- The dredger might not supply enough sand to the sand pumping scheme.
- Due to dam building in the rivers in the proximity of Durban, there might be a shortage of sand supply to the sand trap south of the Durban Harbour breakwater, and thus a shortage of sand supply for the Durban beaches. Initially the sand shortage will manifest as erosion on the Bluff beaches, and once these beaches get close to depletion, the sand supply to the sand trap will not be sufficient.
- The physical characteristics of the sand dredged from the sand trap might also change due to anthropogenic interventions.

Each of these scenarios was investigated individually. With each scenario only the parameter of interest was varied, while all the other parameters were maintained at the baseline scenario settings. A summary of these scenarios is provided in Table 1. The main aim of simulating these future scenarios is to elucidate the consequences of not maintaining the baseline sand supply amount and characteristics (grain size) to the Durban beaches. All the future scenarios were simulated over a period of twenty years, and the results are given in five-year intervals. For all the simulations done in the future scenario study, the initial shoreline was assumed to be the long-term average existing shoreline.

The climate change scenarios were based on a sensitivity analysis approach. Rather than attempting to predict the exact effect of climate change on the incoming wave parameters, the present study aims to investigate the sensitivity of the Durban Bight to changes in only some of the incoming wave characteristics. These types of scenarios can occur if climate change or other physical phenomena change the net movement of weather fronts in the Southern Indian Ocean (Weldon & Reason 2014; Kohfeld

et al 2013; Langlais et al 2011; Kostianoy et al 2004). Current research between the South African Weather Service and the South African Environmental Observation Network is quantifying the magnitude of the expected wave height changes and wave directional approach angle changes. To date these estimates are unclear for the southern African coastlines, and thus a range of scenarios are presented here. A summary of the scenarios investigated in the present study is provided in Table 2. Sea Level Rise (SLR) will ultimately also worsen the effects of the scenarios presented here, as the reach of the waves will extend further inland. Studying the effects of SLR is therefore suggested for future shoreline studies (Habets 2015).

The wave climate changes are applied at the point of input into the Unibest model. In all cases this is before wave breaking is expected to occur and the start of what is known as the dynamic zone. In the dynamic zone the depth to which sediment can move is specified. In areas where larger wave energy is expected (towards the north of the Bight, refer to Figure 2) a deeper contour was chosen as dynamic depth and thus a deeper near-wave breaking depth.

RESULTS

The baseline scenario is depicted in Figure 10. For the most part the baseline scenario maintains the Durban beaches at a relatively stable state. The shoreline does not change much from its initial position. The baseline scenario thus establishes a good reference for the future scenarios.

In Figure 11 the shorelines are given from January 2017 to January 2037 if scenario E(c) were to be implemented every year with an increased wave approach angle (i.e. more northerly or southerly).

In Figure 11 the erosion and/or accretion are not clearly discernible, even with a zoomed-in map view. Scenario E(c) was chosen to illustrate this, because it was one of the scenarios with the greatest effect on the shoreline movement and yet the shoreline dynamics are still not readily observed. To make the change more visible Figure 12 was created. Here the erosion and accretion are illustrated for the entire area of interest as a function of the distance north of Vetch's Reef.

From Figure 12 it is clear where the most erosion and accretion occurred (refer to Figure 2 for beacon locations in the Bight). Since the wave approach angle will cause the coastline to align with it, the most erosion is observed in the corner of the Bight

Table 1 Future sediment pumping volume and sediment characteristics scenario summary (refer to Figure 2)

Pumping volumes and sediment characteristics scenarios				
Discharge point	Baseline (m ³ /year)	A (m ³ /year)	B	C
Booster B1 – 200 m south of Beacon 18	163 968	122 831	Baseline scenario but with D ₅₀ = 333 µm and D ₉₀ = 813 µm	Baseline scenario but with D ₅₀ = 167 µm and D ₉₀ = 407 µm
Booster B2 – Beacon 15	66 223	49 521		
Booster B3 – Beacon 12	7 541	5 509		
Dairy Beach – Beacon 11	20 336	15 106		
Booster B4 – Beacon 7	9 573	7 033		
Total	267 641	200 000		

Table 2 Future climate change scenarios summary

Climate change scenarios			
Discharge point	Baseline (m ³ /year)	D	E
Booster B1 – 200 m south of Beacon 18	163 968	Baseline scenario but with a: a. 5% b. 10% c. 20% increase in significant wave height	Baseline scenario but with a: a. 1° b. 2.5° c. 5° increase and decrease in wave approach direction (TN)
Booster B2 – Beacon 15	66 223		
Booster B3 – Beacon 12	7 541		
Dairy Beach – Beacon 11	20 336		
Booster B4 – Beacon 7	9 573		
Total	267 641		

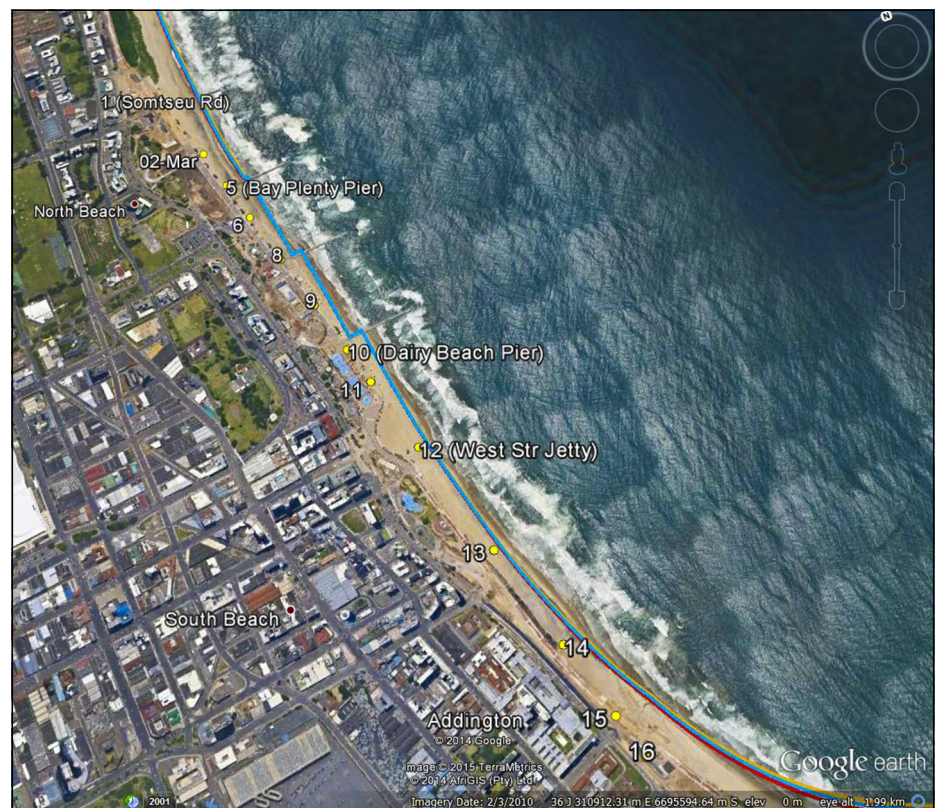


Figure 10 Plan view of baseline scenario together with applicable beacon locations close to the Durban Harbour entrance

(Holthuijsen 2007). To the north accretion is observed as the longshore current deposits the sediment that was removed close to Vetch's Reef and the sediment transport

rates are decreased due to the wave approach angle. It should be noted that this study is only related to long-term shoreline prediction and that cross-shore processes

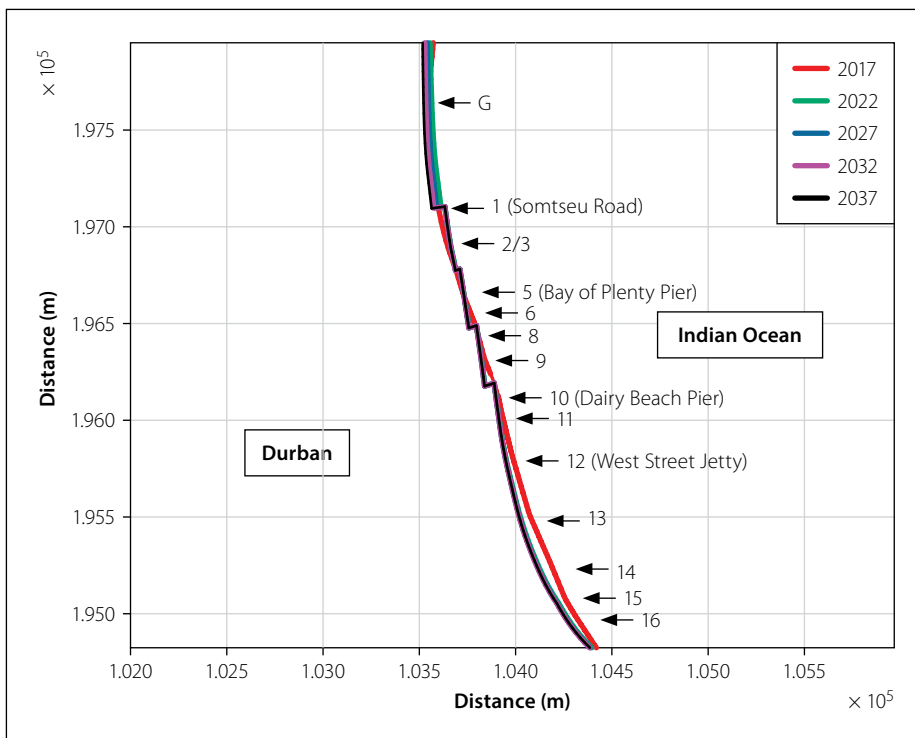


Figure 11 Plan view of scenario E(c) with increasing wave angle (refer to Table 2)

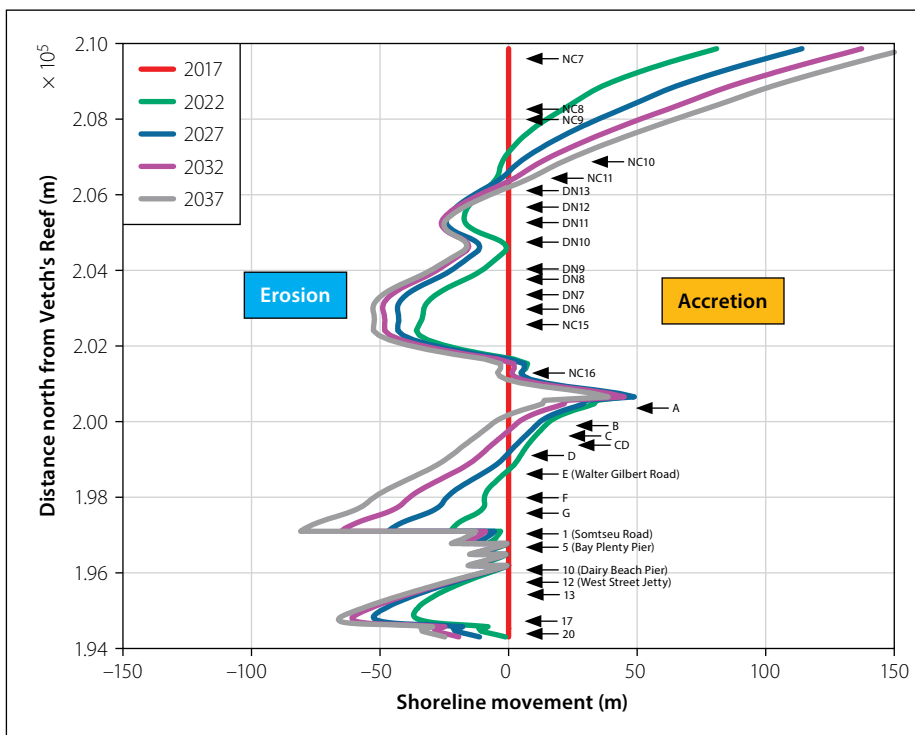


Figure 12 Total erosion and accretion for the entire area of interest

are not considered. The results, especially between the piers, should be interpreted with care. The shoreline stability between the piers also indicate the effectiveness of these structures in slowing down structural erosion (Bosboom & Stive 2015). The groynes close to beacon A and DN10 also caused local accretion on the down-drift side of these structures.

Figure 12 and Figures 13(a) to (k) give all the scenarios summarised in Tables 1 and 2.

DISCUSSION

In Figure 13(a) the scenario is simulated of pumping 25% less sand to the Bight. As expected, erosion occurred especially in the area at the down-drift side of the piers. After twenty years a maximum erosion in the order of 40 m is expected. Erosion is first expected at the southern side of the model, while erosion to the northern part of the model is expected once the southern beaches are depleted of sediments. The stabilisation effect of the piers is again clear

in this simulation. In the model the beach width is not constrained, but in reality significant areas of the Bight beaches are backed by hard structures along their landward edges (e.g. by the promenade, parking areas, revetments, etc). Thus, the model may in certain cases predict potential shoreline erosion extending further landward than the actual extent of the existing sandy beach area. In such instances the actual shoreline response will necessarily differ from the simulated response (also depending on the backshore area and possible preemptive intervention measures implemented by the municipality).

In Figure 13(b) the scenario with an increase in sediment size is depicted. In this case accretion is observed in the area close to where the pumping takes place. Further to the north of the Bight erosion is observed, as the heavier sediment cannot readily be transported to this area by means of the longshore currents. The inverse is observed in Figure 13(c), as the finer sediment assumed in scenario C is readily transported northward, thus causing erosion in the corner of the Bight and accretion further northward due to the presence of groynes. In most of these results some areas reach the new equilibrium more rapidly than other areas.

If consecutive years' predictions overlap, it implies that those areas' shorelines have reached its new equilibrium position. From a coastal management point of view these areas will require the least amount of mitigation and/or intervention to maintain future beach stability. In areas where continued erosion or accretion is observed, the gradient of erosion and accretion is still present over time. A decreasing steepness in the gradient is an indication that the shoreline is realigning itself towards its new equilibrium position. The simulations were not run long enough to illustrate the total new equilibrium position of the entire coastline, but it was assumed that the gradient in erosion and accretion provides useful additional assistance to coastal management and governing authorities, for example to identify the areas where the greatest potential future shoreline impacts are expected. The new beach equilibrium may never be reached in the model. In reality the beach profile will flatten and the shallow water waves will dissipate differently towards the shore. This is a limitation of the model that must also be kept in mind, and the beach profile used must be updated with large erosion and/or accretion events.

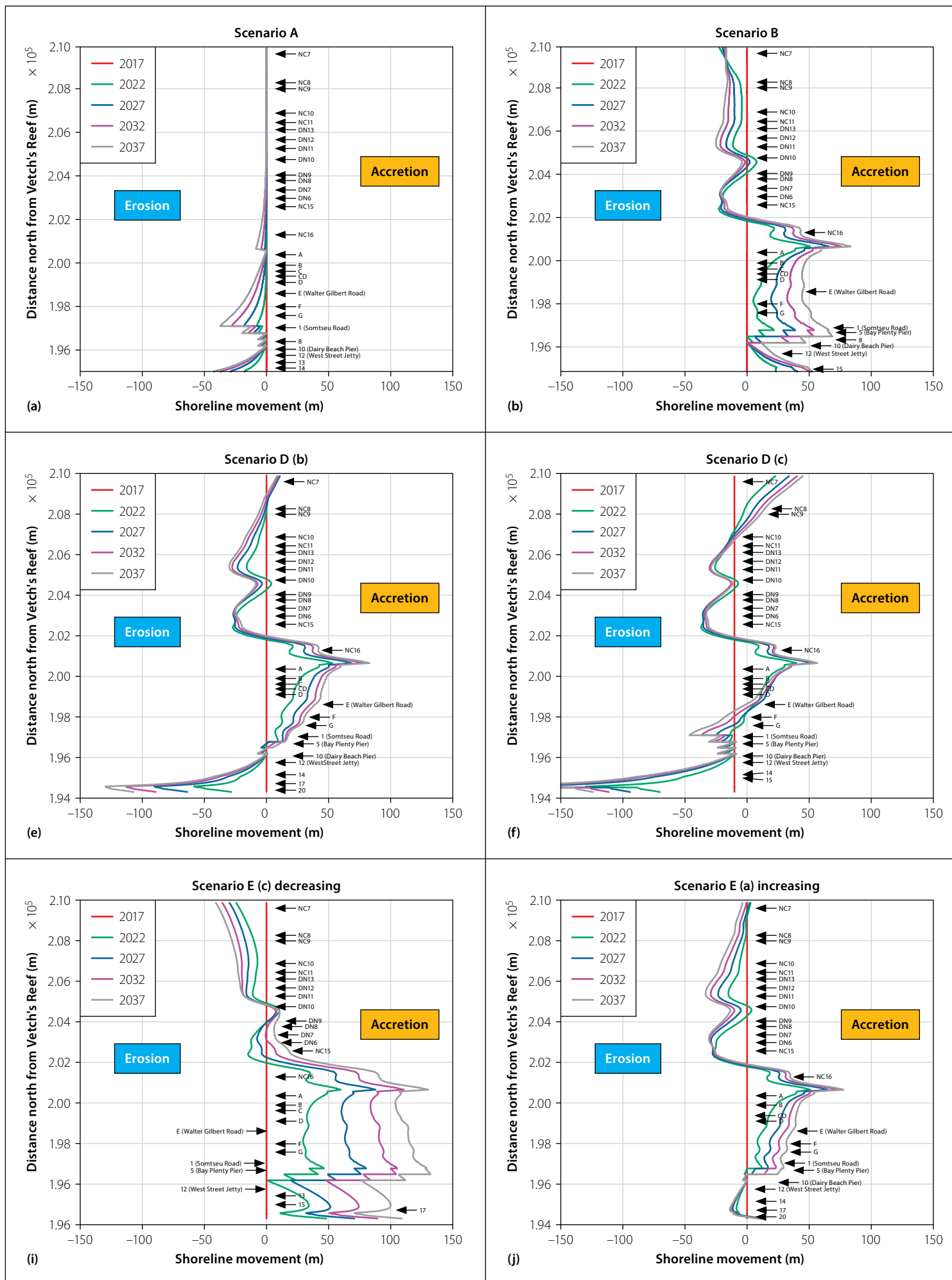
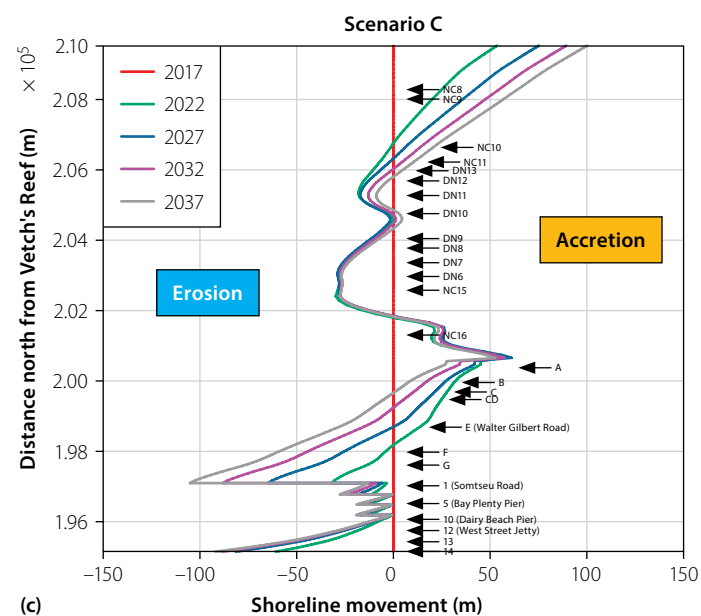
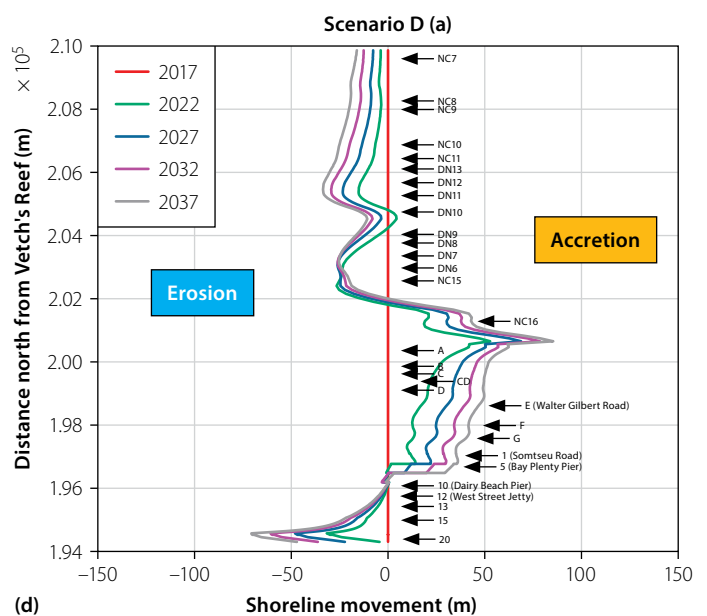


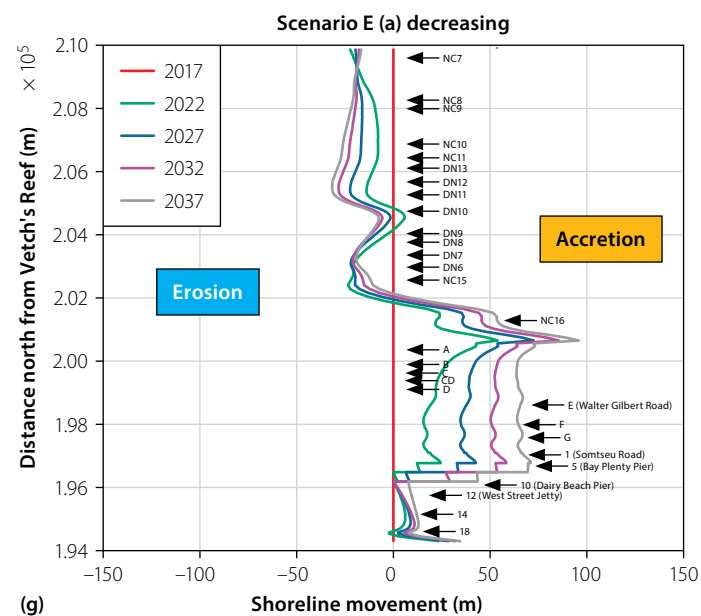
Figure 13(a)–(k) Results of long-term numerical modelling grouped per scenario, as described in Tables 1 and 2; the title of each figure describes the scenario



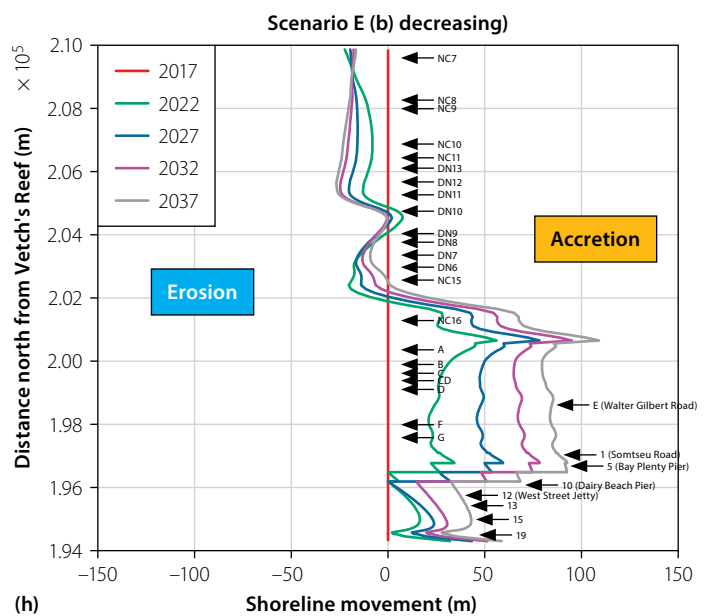
(c)



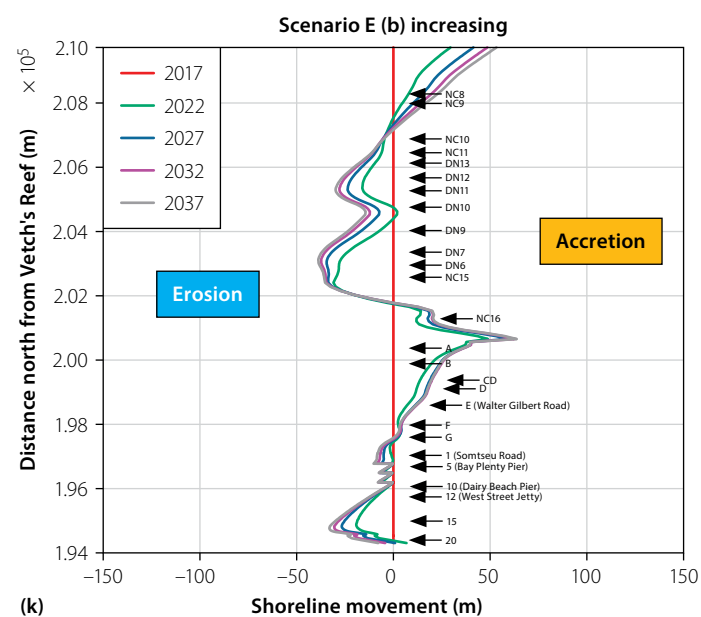
(d)



(g)



(h)



(k)

In Figures 13(d), (e) and (f) an increase in wave height has the net effect of erosion in the corner of the Bight. The increase in wave height can be related to an increase in wave power, and thus the erosion potential increases on the beaches where the longshore current is strengthened. All three percentage increases under scenario D had the same outcome over a twenty-year period. The only aspect that differed was the rate at which the final coastline was reached. The higher the wave energy, the quicker a seemingly new stable coastline was obtained. The area down-drift of the piers, experiencing erosion, also eroded more, and the extent of the erosion increased with an increase in wave height. The amount of accretion observed at the northern extent of the model also increased with increase in wave height. This is a sign that the amount of northwardly transported sediment is increased to such an extent that the sediment will start to build up on the northern beaches. The amount of accretion caused by the groynes also decreased with increase in wave energy.

In Figures 13(g) to (i) the effects of a decreasing wave approach angle (anti-clockwise) is illustrated. Here it can be seen how the waves are reshaping the beaches to adapt to the new approach angle.

corresponding to the table descriptions

The net effect of this scenario is accretion in the corner of the Bight and erosion at the northern beaches. The locations of the erosion and accretion are associated with the increase and decrease in the longshore transport rate associated with the change in wave approach angle.

For the last scenario, the opposite change in wave approach angles was investigated and is depicted in Figures 13(j) and (k) and Figure 12. In this scenario accretion is observed with an increase of one degree in wave approach angle (clockwise). For an increase of two and a half degrees a new equilibrium scenario is reached from 2022 onwards. If the approach angle is increased even further to five degrees (scenario E(c)) progressive erosion is observed. This scenario illustrates some similarities to the increasing wave height scenario D, with similar accretion to the north of the Bight. An increasing wave approach angle of two and a half degrees is thus a critical point between a future progressively eroding or accreting coast (in the southern corner of the Bight for scenarios with an increasing wave approach angle).

The results of the present study illustrate the relative changes in future shorelines based on possible future scenarios. These results must be interpreted as indicative and may be used as a decision support tool for the management of the beaches at Durban. The exact amount of erosion and accretion observed in the future might differ from the results presented here, depending on the actual changes in Southern and Indian ocean (and atmospheric) dynamics (considering model inaccuracies). Once more research is available to quantify these changes, the present study scenario closest to the predicted offshore changes can be readily used by coastal managers. The present study also identifies the areas where potential future intervention might be required, and thus helps coastal managers in planning where to focus their attention and which valuable coastal infrastructure would be most threatened. The potential severity of some of the predicted results should not be underestimated. With dry beach widths along the Durban Bight currently ranging from as little as about 15 m up to about 90 m (measured on Google Earth imagery of September 2017), very little or not much buffer is available on the Durban beaches to tolerate several of the erosion predictions. Erosion of 40 m or 50 m of the upper beach would be catastrophic in many Bight

areas, and therefore require earlier sand nourishment if such progressive erosional trends are observed.

A small accreting trend (approximately 1 m per annum) was also observed in both the model and the data at most of the monitoring beacons (refer to Figure 6 for historical records). Analysing these trends fell outside the scope of the current study and is therefore suggested for future research.

CONCLUSION

The shoreline dynamics of the Durban Bight were investigated using numerical models. Calibration and validation were done using existing historical data. The accuracy of the model predictions was quantified using the RMSEP. Once the model was adequately calibrated, the possible future scenarios were simulated. Each parameter was investigated by varying only that parameter and keeping the rest of the input criteria identical to the baseline scenario.

The effects of the four future scenarios were clear. These results were compared with the baseline scenario in which the beaches remained reasonably stable for the twenty-year simulation period. The effect of reducing the sand-pumping volumes was a reduction in beach width. The erosion was particularly extensive in the area between the harbour entrance and the piers. The erosion lessened further north in the Bight, but it can be expected that these beaches will also erode once the sand supply from the southern beaches diminishes in time. In all the future scenarios the piers seemed to reduce the amount of erosion of the beaches resulting from longshore transport.

The other two scenarios involved the alteration of the grain size of sand being pumped onto the Durban beaches. A general increase of 25% in grain size, as well as a decrease of 25% in grain size, had the total effect of erosion on the Bight beaches. The way the erosion occurred was different in that the finer sand particles eroded quickly and then remained relatively stable, while the coarser sand eroded consistently. The dynamics south and north of the piers were also different in both cases, with the finer lighter particles indicating more extensive erosion south and north of the piers.

The effect of climate change might be either erosion or accretion, depending on

the scenario and beach location. It is thus also clear that more research is needed to quantify the exact directional and wave energy changes southern Africa can expect in the future. These investigations should also include sea level rise (SLR), as this will increase the reach of the prevailing wave climate and bring about cross-shore beach profile shifts. Given more accurate climatological prediction, the appropriate shoreline evolution scenario may be utilised in the Durban coastal management plan, both to protect existing infrastructure and to design and build new infrastructure with an appropriate setback distance. This study may also inform future studies regarding the general sand budget of the east coast of South Africa, which includes the building of more dams on the sediment-supplying rivers of the east coast. The present study indicated which areas of the Bight were least susceptible to severe shoreline changes, and thus assists the governing authorities in focusing their management strategies to the areas most at risk of future shoreline changes. With knowledge of such potential changes, the eThekweni Municipality will be able to investigate ways to manage and mitigate the potential impacts of wider/narrower beaches, such as blockage of stormwater outlets, windblown sand problems, reduced available beach space for recreational activities and possible effects on the Umgeni River mouth.

Generally, small alterations of the Bight conditions may lead to adverse effects, and thus maintaining the current sand-pumping rates and sediment characteristics are of utmost importance.

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