Effect of the minimum void ratio on the vertical intercept of the steady state line of non-plastic soils

L A Torres-Cruz, S Geyer, P R Mackechnie

The steady state line (SSL) plays a key role in understanding and modelling the mechanical response of soils. Accordingly, understanding how the SSL correlates to soil index properties is of primary importance. A previous study reported that the vertical location of the SSL (Γ_1) in void ratio (e) versus mean effective stress (p') space is correlated to the minimum void ratio (e_{min}). However, the correlation only included soils with narrow particle size distributions (PSD) and low fines content (FC). In the current study, published data corresponding to 30 non-plastic soils were re-processed to further explore the applicability of the Γ_1 - e_{min} correlation. The results indicate that the Γ_1 - e_{min} correlation is linear (R^2 = 0.85) and valid regardless of the coefficient of uniformity (C_u), FC, and particle shape. The Γ_1 - e_{min} dataset presented herein was also compared to a previously published dataset, and good agreement was observed. It is proposed that the Γ_1 - e_{min} correlation can be very useful to understand how the Γ_1 of different non-plastic soils compare to one another, and to minimise the extent of triaxial testing required when characterising a soil deposit from an SSL standpoint. Limitations of the Γ_1 - e_{min} correlation are also discussed.

INTRODUCTION

Soils reach constant values of void ratio (*e*), mean effective stress ($p' = (\sigma'_1 + \sigma'_2 + \sigma'_3)/3$), and deviator stress ($q = \sigma'_1 - \sigma'_3$) when sheared to large strains (Castro 1969). The locus of steady state (e, p', q) coordinates attained when shearing from different initial states, defines the steady state line (SSL). Because the SSL represents the stress and void ratio conditions towards which a soil evolves when sheared, it plays a key role in defining mechanical response. Two projections are typically used to define the SSL: q-p' (stress plane) and q-p' (compression plane). The

q-p' projection, which reflects steady state frictional properties, is strongly dependent on particle shape (Cho *et al* 2006) and largely independent of particle size distribution (PSD) (Carrera *et al* 2011; Rahman *et al* 2014). By contrast, the *e-p'* projection, representative of stiffness, is affected by particle shape (Cho *et al* 2006), PSD (Thevanayagam *et al* 2002; Rahman & Lo 2008; Muir-Wood & Maeda 2008; Li *et al* 2013), and void ratio limits (Cho *et al* 2006; Cubrinovski & Ishihara 2000; Hemer *et al* 2016). Given the greater number of factors that affect the *e-p'* projection, it has received attention from a significant number

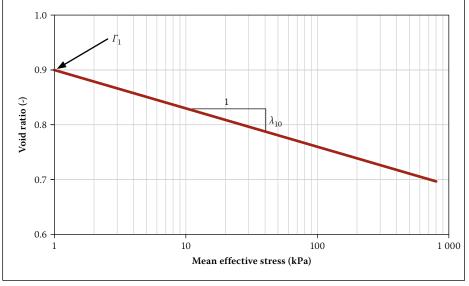


Figure 1 Idealisation of the SSL using Equation 1

TECHNICAL NOTE

JOURNAL OF THE SOUTH AFRICAN INSTITUTION OF CIVIL ENGINEERING

ISSN 1021-2019

Vol 59 No 2, June 2017, Pages 59-64, TN-77



DR LUIS ALBERTO TORRES-CRUZ obtained his BSc in Civil Engineering from the Universidad del Valle in Cali, Colombia, and completed his PhD in Geotechnical Engineering at the University of the Witwatersrand (Wits). His doctoral research focused on the use of the cone penetration test to assess the liquefaction potential of tailings dams. He is currently a

lecturer in Geotechnical Engineering at Wits.

Contact details:
Department of Civil and Environmental Engineering
University of the Witwatersrand
Private Bag 3
Wits
2050
South Africa

E: Luis Alberto. Torres Cruz@wits.ac.za



T: +27 11 717 7150

STEPHAN GEYER obtained his BSc in Civil Engineering from the University of the Witwatersrand. He is currently employed at Prime Resources Environmental Consultants, where he is involved in the geotechnical and design aspects of water and mining residue management, including tailings storage facilities

Contact details:
Prime Resources Environmental Consultants
PO Box 2316
Parklands
2121
South Africa
T: +27 11 447 4888
E: steohan@resources.co.za



PETER MACKECHNIE obtained his BSc degree in Civil Engineering from the University of the Witwatersrand.

Contact details:
Department of Civil and Environmental Engineering
University of the Witwatersrand
Private Bag 3
Wits
2050
South Africa
T: +27 11 326 0030
E: mackechniep@gmail.com

Keywords: steady state line, critical state line, minimum void ratio, non-plastic soils of researchers and is the focus of this note. The *e-p*' projection is commonly modelled with Equation 1:

$$e = \Gamma_1 - \lambda_{10} \log_{10}(p') \tag{1}$$

Where λ_{10} is the slope of the SSL in semilogarithmic space and Γ_1 is the void ratio at p'=1 kPa (Figure 1). The current note will explore the correlation between the minimum void ratio (e_{min}) , which is associated with a defined maximum density state, and Γ_1 .

Several studies have investigated how the SSL is affected by soil index properties. Thevanayagam et al (2002) tested gapgraded mixtures composed of sand and nonplastic fines and concluded that the vertical location of the SSLs could be explained by the fines content (FC). They noted that as FC increases from zero, the SSL shifts downwards in e-p' space (Γ_1 decreases), and that beyond a certain FC value it shifts upwards (Γ_1 increases). The FC value at which the SSL shift reverses direction was termed by Thevanayagam et al (2002) as the threshold FC (TFC). The calculation of the parameters proposed by Thevanayagam et al (2002) to explain the effect of FC required knowledge of the SSLs of the different sand-fines mixtures. Consequently, the framework lacked predictive power. To overcome this, Rahman & Lo (2008) developed semi-empirical equations to calculate, as a function of FC and other PSD descriptors, the parameters in the framework proposed by Thevanayagam et al (2002). This allowed the prediction of the SSL of sands with FC < TFC, provided that the SSL of another sand-fines mixture with FC < TFC was known. More recent works show that the effect of non-plastic fines on the SSL continues to be investigated (e.g. Mohammadi & Qadimi 2015; Rahman et al 2014; Yang et al 2015).

Despite the success of Rahman & Lo (2008) in predicting several SSLs, their framework has limitations that hinder its wider applicability. For example, the framework is limited to soils with FC smaller than the TFC, which tends to be close to 40%. Additionally, the framework cannot explain the differences between the SSLs of soils with no fines but different PSDs or grain shape.

Cho *et al* (2006) explored the correlation between Γ_1 and e_{min} of 49 natural and crushed sands with mostly little to no fines (only six sands had FC > 12%) and a maximum coefficient of uniformity (C_u) of 6.2 (only three sands had C_u > 4). They found that the correlation was linear, independent of particle shape, and of modest strength (R^2 = 0.54). However, the validity of the Γ_1 - e_{min} correlation for high FC values or widely graded soils remains untested. The objective of this

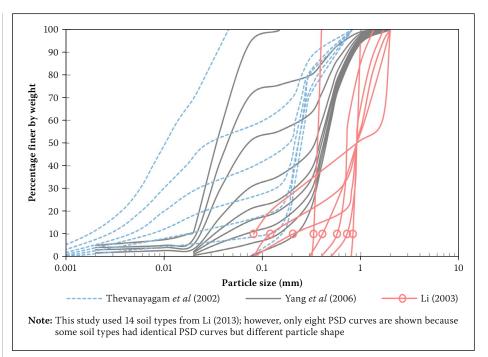


Figure 2 PSD curves of the 30 soils whose data was processed by the current authors

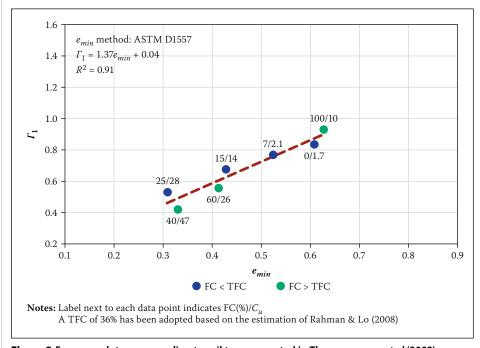


Figure 3 Γ_1 vs e_{min} plot corresponding to soil types reported in Thevanayagam *et al* (2002)

note is to present evidence that expands the applicability of the Γ_1 - e_{min} correlation to a wide range of non-plastic soils, regardless of PSD descriptors such as FC and C_u . The wide applicability of this correlation is considered a step towards overcoming the limitations encountered when using FC and TFC to explain the location of the SSL.

METHODOLOGY

Datasets from three references were processed to explore the validity of the Γ_1 - e_{min} correlation: Thevanayagam et~al~(2002), Yang et~al~(2006) and Li (2013). The dataset from Thevanayagam et~al~(2002) includes seven soils composed of foundry sand mixed with

non-plastic crushed silica fines. The resulting mixtures have FCs varying from 0% to 100% and C_{ij} varying from 1.7 to 47. The dataset from Yang et al (2006) includes nine soils composed of Hokksund sand mixed with non-plastic Chengbei silt. The resulting mixtures have FCs varying from 0% to 94% and C_{ij} varying from 2 to 14. The Thevanayagam et al (2002) and Yang et al (2006) datasets were used herein to assess the validity of the Γ_1 - e_{min} correlation over a wide range of FC and C_u values. The dataset from Li (2013) includes 14 soils of which six were made of glass balls (spherical particles) and eight of Hostun sand (angular particles). Two soils, one each of glass balls and Hostun sand, had FC =10%, whereas the remaining 12 had

Table 1 Values of p' and e used to calculate Γ_1

Soil type	p' (kPa)	e	Soil type	p' (kPa)	e	Soil type	p' (kPa)	e	Soil type	p' (kPa)	e
SIM ^a (0/1.7) ^b	5	0.801	SIM (100/10)	13	0.854	GB (0/2.5)	277.9	0.577	HCM (20/13)	83	0.534
	10	0.796		157	0.821		543.2	0.566		115	0.560
	23	0.774		315	0.767		840.3	0.559		221	0.543
	41	0.774	HS ^d (0/1.1/0.9) ^e	25.9	0.779	- GB (0/5)	131.6	0.470		268	0.556
	48	0.765		162.5	0.732		268.6	0.460		515	0.482
	157	0.746		328.3	0.719		360.4	0.462		0.1	0.626
	473	0.718		639.0	0.700		546.0	0.448		0.1	0.612
	937 ^c	0.668	HS (0/1.1/0.35) ^e HS (0/1.4/0.9) ^e	158.9	0.730	GB (0/10) - GB (10/20)	136.6	0.421		0.5	0.603
	937	0.645		315.3	0.706		144.8	0.413	- НСМ (30/14)	0.7	0.588
	1 099	0.681		632. 6	0.697		271.1	0.404		1.0	0.532
	1 217	0.595		11.5	0.780		572.6	0.392		22	0.477
	1 407	0.606		158.4	0.726		75.1	0.342		108	0.544
SIM (7/2.1)	5	0.731		159.2	0.727		131.0	0.332		209	0.536
	14	0.717		160.6	0.723		274.6	0.321		272	0.530
	216	0.670		224.3	0.720		579.1	0.304		528	0.448
	236	0.665		249.5	0.720	HCM ^g (0/2.4)	14	0.854	HCM (50/8.9)	0.1	0.758
	434	0.628		316.4	0.713		29	0.850		0.4	0.727
	547	0.595		497.5	0.700		77	0.842		1.2	0.746
	1 234	0.553		544.1	0.690		111	0.828		1.8	0.667
SIM (15/14)	1	0.617		629.9	0.691		215	0.825		3	0.635
	17	0.589	HS (0/1.4/0.75) ^e	165.3	0.723		268	0.807		109	0.683
	42	0.575		318.1	0.710		335	0.809		139	0.567
	94	0.600		627.1	0.698	HCM (5/3.4)	9	0.789		165	0.523
	124	0.575	- HS (0/2.5)	9.6	0.774		54	0.771		208	0.673
	306	0.522		12.8	0.770		98	0.781		262	0.673
	306	0.513		61.3	0.743		121	0.779	HCM (70/2.2)	1.0	0.981
	1 289	0.421		159.7	0.714		236	0.760		1.3	1.006
SIM (25/28) SIM (40/47)	1	0.477		321.3	0.695		275	0.756		2.2	0.958
	11	0.461		640.5	0.675		306	0.752		3	0.815
	69	0.430	HS (0/5)	162.1	0.694	HCM (10/6.6)	51	0.715		4	0.790
	73	0.416		323.4	0.671		64	0.693		12	0.748
	83	0.430		326.3	0.681		111	0.675		105	0.900
	232	0.404		644.1	0.662		115	0.709		126	0.723
	690	0.357	HS (0/10)	164.7	0.618		221	0.650		194	0.877
	787	0.357		324.7	0.608		286	0.665		253	0.885
	4	0.410		652.0	0.606		495	0.679		0.7	1.242
	7	0.401	HS (10/20) - GB ^f (0/1.1)	164.5	0.602	- НСМ (15/11)	7	0.678		2.3	1.267
	13	0.390		325.1	0.595		7	0.670		2.3	1.219
	64	0.390		645.2	0.586		61	0.658		4	0.983
	265	0.363		138.1	0.671		70	0.636		5	1.094
	5	0.547		275.6	0.663		109	0.633		6	1.046
	5	0.530		556.5	0.655		226	0.607		97	1.150
	11	0.539		1 363.7	0.645		283	0.615		152	0.929
SIM (60/26)	112	0.514	GB (0/1.4)	119.9	0.643		314	0.584		184	1.125
	138	0.514		252.6	0.633	HCM (20/13)	1.4	0.650		240	1.113
	157	0.511		498.3	0.623		2.5	0.638			
	184	0.497		1 254.8	0.621		7	0.624]		
SIM (100/10)	10	0.877	GB (0/2.5)	138.2	0.586		32	0.579			-

a. SIM = Silica sand-silt mixtures tested by Thevanayagam $et\ al\ (2002)$

b. Values in parentheses indicate FC(%)/ C_u c. Values of p' and e that appear in bold-italic font were not used to calculate Γ_1 as they could not be adequately fitted with Equation 1 d. HS = Hostun sand tested by Li (2013)

e. Third value inside the parentheses indicates the mean grain size in mm f. GB = Glass balls tested by Li (2013)

g. HCM = Mixtures of Hokksund sand and Chengbei silt tested by Yang et al (2006)

FC = 0%. C_u varied from 1.1 to 20. This dataset was used herein because: (i) some of the soils made up of glass balls had considerably low e_{min} values which allowed a significant extension of the lower bound of the domain of the Γ_1 - e_{min} correlation; (ii) given that the particle shape of the glass balls is distinctly different from that of Hostun sand, this dataset enables a straightforward assessment of whether particle shape affects the Γ_1 - e_{min} correlation; and (iii) this dataset also allows assessment of the validity of the Γ_1 - e_{min} correlation at different C_u values. Figure 2 presents the PSDs of the 30 soils considered.

Values of Γ_1 were calculated by fitting Equation 1 to the (p', e) points that defined the SSL of each soil (Table 1). Some of the SSLs reported by Thevanayagam et al (2002) cannot be modelled with Equation 1, due to the curvature of the SSL in e-log₁₀p'which some soils exhibit at high stress levels (e.g. Been et al 1991; Li & Wang 1998). Consequently, some (p', e) points with high p' values were excluded from the fitting process. Similarly, given the experimental difficulties and uncertainties involved in performing triaxial tests at very low values of effective stress, two (p', e) points with p' = 1 kPa were also disregarded when calculating Γ_1 (see footnote c in Table 1).

As annotated in Figures 3 to 5, different methods were used to determine the e_{min} values of each dataset. ASTM D1557 refers to the modified Proctor compaction test, and ASTM D4253 refers to the method of soil densification using a vibratory table. Although the current authors acknowledge that e_{min} values from different methods are not strictly comparable, it is hypothesised that, regardless of the method, the resulting e_{min} provides a reasonable indicator of packing efficiency.

RESULTS AND DISCUSSION

Figures 3 to 5 suggest strong ($R^2 \ge 0.90$) linear Γ_1 - e_{min} correlations. The data point labels further suggest that the correlations are valid regardless of FC or C_{ν} . The independence of the correlation from FC observed in Figures 3 and 4 is at odds with previous works (e.g. Thevanayagam et al 2002; Rahman & Lo 2008; Rahman et al 2014) which have suggested that Γ_1 is fundamentally correlated to FC. Furthermore, Figure 3 explicitly shows that essentially the same Γ_1 - e_{min} correlation is followed regardless of whether FC is smaller or greater than TFC. Additionally, the angular Hostun sand and the glass balls follow the same Γ_1 - e_{min} correlation despite their significantly different particle shapes (Figure 5). This result agrees with Cho et al (2006) who reported

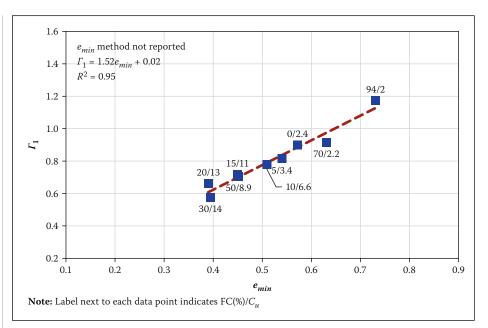


Figure 4 Γ_1 vs e_{min} plot corresponding to soil types reported in Yang *et al* (2006)

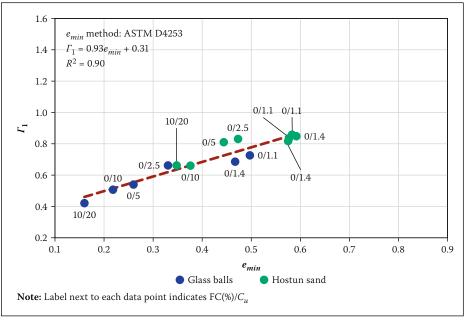


Figure 5 Γ_1 vs e_{min} plot corresponding to soil types reported in Li (2013)

the independence from particle shape of the Γ_1 - e_{min} correlation for narrowly graded sands with low values of FC.

When all 30 soils are collectively plotted (Figure 6), a single linear correlation emerges ($R^2 = 0.85$). The slight decrease in \mathbb{R}^2 (compare to Figures 3 to 5) is likely a consequence of combining SSLs calculated from triaxial tests conducted in different laboratories and following slightly different protocols, and e_{min} values obtained through different procedures. For example, Prochaska & Drnevich (2005) showed that the maximum dry unit weight, which is associated to e_{min} , can show variances of the order of ± 3% when estimated from different compaction techniques. A unique linear correlation $(R^2 = 0.77)$ continues to be apparent when the data corresponding to Cho et al (2006) is included (Figure 7). This indicates that

the Γ_1 - e_{min} correlation observed by Cho et al (2006) in narrowly graded sands with low FC values is approximately the same for soils with significant amounts of non-plastic fines and large C_u values, such as those represented in Figures 3 to 5. The data analysed in this study has also been useful to expand the lower bound of the domain of the Γ_1 - e_{min} correlation reported by Cho et al (2006) (Figure 7).

The authors suggest that the validity of the Γ_1 - e_{min} correlation over such a wide range of non-plastic soil types is explained by the similarity in which both Γ_1 and e_{min} are affected by a soil's fundamental properties. For example, they are both directly correlated to particle angularity (Li 2013; Biarez & Hicher 1994; Cho et~al~2006), inversely correlated to C_u (Li 2013; Biarez & Hicher 1994; Poulos et~al~1985), and respond in a similar

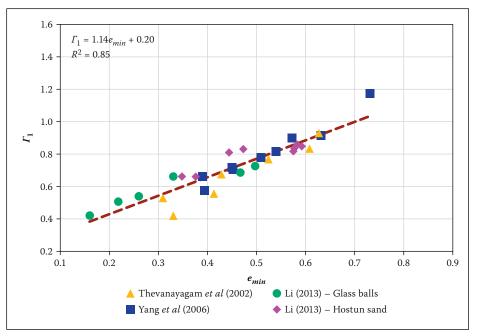


Figure 6 Γ_1 vs e_{min} plot including all the data points from Figures 3 to 5

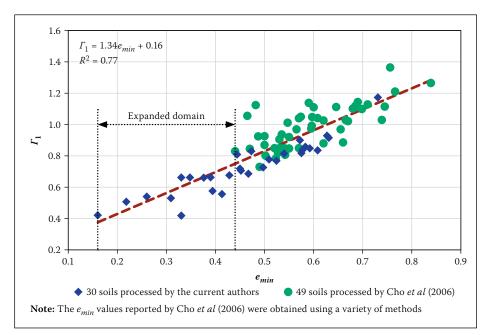


Figure 7 Γ_1 vs e_{min} plot including all the data points from Figures 3 to 5 and the data points from Cho *et al* (2006)

manner to changes in FC (Lade et~al 1998; Rahman & Lo 2008). It is also important to acknowledge that the use of e_{min} as a predictor of Γ_1 may be limited by the differences between the remoulding and particle crushing mechanisms that a soil undergoes when sheared to steady state and when compacted to e_{min} .

The authors are not recommending the use of the correlations in Figures 3 to 7 to replace triaxial testing to determine the SSL, as doing so can result in significant errors. For example, in Figure 7 a deviation from the best fit line of \pm 0.1 is observed in Γ_1 , implying a potential error of 0.2. This value is significantly higher than the error of \pm 0.01 in void ratio, which was suggested by Jefferies & Been (2006) as a reasonable

target error when calculating the SSL experimentally. Notwithstanding these potential errors, the correlation is very useful to qualitatively understand how the Γ_1 values of different non-plastic soil types will compare to one another. Additionally, when studying a heterogeneous soil deposit that includes a variety of soil types, the correlation can help reduce the number of soil types whose SSLs have to be experimentally determined to fully characterise the deposit from an SSL standpoint (e.g. Hemer $et\ al\ 2016$).

CONCLUSIONS

The Γ_1 - e_{min} correlation of 30 non-plastic soils has been investigated. The results indicate that the correlation is linear ($R^2 = 0.85$)

and valid regardless of FC, C_u , and particle shape. Comparison of the results presented herein (Figure 6) with the Γ_1 versus e_{min} dataset obtained by Cho et~al~(2006), indicates that the Γ_1 - e_{min} correlation originally observed by Cho et~al~(2006) for narrowly graded sands with small amounts of fines may be applicable to all non-plastic soils (Figure 7). The similarity in the way in which both Γ_1 and e_{min} respond to changes in the fundamental properties of a soil is believed to be the reason why the Γ_1 - e_{min} correlation is valid over a wide variety of soil types.

Given that the effect of non-plastic fines on the SSL continues to be intensely researched, it is an important finding of this work that the $\varGamma_1\text{-}e_{min}$ correlation is not affected by FC (Figures 3 and 4). This is not entirely surprising though, as it seems unlikely that a fundamental soil property (Γ_1) can be explained by a property such as FC which is arbitrarily defined as the percentage of particles smaller than 75 μm (or 63 μm, depending on the standard). Accordingly, it must be expected that the predictive power of FC and associated concepts like the TFC will have important limitations. The results presented herein suggest that a better understanding of the SSL can be achieved by correlating Γ_1 to e_{min} rather than to FC. The authors are currently analysing an extended database and conducting triaxial experiments to continue exploring the strengths and limitations of the Γ_1 - e_{min} correlation.

ACKNOWLEDGEMENTS

The authors gratefully acknowledge Dr Nico Vermeulen (Jones & Wagener) and Dr Irvin Luker (University of the Witwatersrand) for their valuable insights and discussions.

REFERENCES

Been, K, Jefferies, M G & Hachey, J E 1991. The critical state of sands. *Geotechnique*, 41(3): 365–381.

Biarez, J & Hicher, P-Y 1994. Elementary Mechanics of Soil Behaviour: Saturated Remoulded Soils. Rotterdam: Balkema.

Carrera, A, Coop, M & Lancellota, R 2011. Influence of grading on the mechanical behaviour of Stava tailings. *Geotechnique*, 61(11): 935–946.

Castro, G 1969. *Liquefaction of sands*. PhD thesis, Cambridge, MA: Harvard University.

Cho, G, Dodds, J & Santamarina, J 2006. Particle shape effects on packing density, stiffness, and strength: Natural and crushed sands. Geotechnical and Geoenvironmental Engineering, 132(5): 591–602.

Cubrinovski, M & Ishihara, K 2000. Flow potential of sandy soils with different grain compositions. *Soils and Foundations*, 40(4): 103–119.

Hemer, J C, Mincione, N L M & Torres-Cruz, L A 2016. Determination of steady state lines for

- non-plastic platinum tailings. In: Jacobsz, S W. (Ed.), Proceedings, 1st Southern African Geotechnical Conference Held at Sun City, 4–6 May 2016. London: CRC Press/Taylor & Francis, 275–280.
- Jefferies, M G & Been, K 2006. Soil liquefaction: A critical state approach. London: Taylor & Francis.
- Lade, P V, Liggio, C & Yamamuro, J A 1998. Effects of non-plastic fines on minimum and maximum void ratios of sand. Geotechnical Testing Journal, 21(4): 336–347
- Li, G 2013. Étude de l'influence de l'étalement granulometrique sur le comportement mecanique des materiaux granulaires. PhD thesis, Nantes, France: Ecole Central de Nantes [In French].
- Li, G, Ovalle, C, Dano, C & Hicher, P-Y 2013. Influence of grain size distribution on critical state of granular materials. In: Yang, Q, Zhang, J-M, Zheng, H & Yao, Y (Eds.), Constitutive Modeling of Geomaterials, Berlin, Heidelberg: Springer Verlag.
- Li, X S & Wang, Y 1998. Linear representation of steady state-line for sand. *Geotechnical and Geoenvironmental Engineering*, 124(12): 1215–1217.
- Mohammadi, A & Qadimi, A 2015. A simple critical state approach to predicting the cyclic and

- monotonic response of sands with different fines contents using the equivalent intergranular void ratio. *Acta Geotechnica*, 10(5): 587–606.
- Muir-Wood, D & Maeda, K 2008. Changing grading of soil: Effect on critical states. *Acta Geotechnica*, 3(1): 3–14
- Poulos, S J, Castro, G & France, J W 1985. Liquefaction evaluation procedure. *Geotechnical Engineering*, 111(6): 772–792.
- Prochaska, A B & Drnevich, V P 2005. One-point vibrating hammer compaction test for granular soils. *Proceedings*, ASCE Geo-Frontiers Conference held in Austin, TX, 24–26 January. Reston, VA: ASCE.
- Rahman, M M & Lo, S R 2008. The prediction of equivalent granular steady state line of loose sand with fines. *Geomechanics and Geoengineering*, 3(3): 179–190
- Rahman, M M, Lo, S R & Dafalias, Y F 2014. Modelling the static liquefaction of sand with low-plasticity fines. *Geotechnique*, 64(11): 881–894.
- Thevanayagam, S, Shenthan, T, Mohan, S & Liang, J 2002. Undrained fragility of clean sands, silty sands, and sandy silts. *Geotechnical and Geoenvironmental Engineering*, 128(10): 849–859.

- Yang, S L, Sandven, R & Grande, L 2006. Steady-state lines of sand–silt mixtures. *Canadian Geotechnical Journal*, 43(11), 1213–1219.
- Yang, J, Wei, L M & Dai, B B 2015. State variables for silty sands: Global void ratio or skeleton void ratio? Soils and Foundations, 55(1): 99–111.

NOTATION

e = void ratio

 $p' = \text{mean effective stress } (\sigma'_1 + \sigma'_2 + \sigma'_3)/3$

 $q = deviator stress (\sigma'_1 - \sigma'_3)$

SSL = steady state line

 Γ_1 = steady state void ratio that corresponds to a mean effective stress of 1 kPa

 $\lambda_{10} = \text{slope of the SSL in e-log}_{10}(p') \text{ space}$

FC = fines content ($< 75 \mu m$)

PSD = particle size distribution

 e_{min} = minimum void ratio

TFC = threshold fines content

 C_u = coefficient of uniformity (D₆₀/D₁₀)